# Illinois Structural Masonry Coalition



# Ninety 7 Fifty On The Park

### NINETY 7 FIFTY ON THE PARK

Orland Park, Illinois

#### **ARCHITECT**

RTKL Associates Inc.

#### STRUCTURAL ENGINEER

Samartano & Company

#### **GENERAL CONTRACTOR**

James McHugh Construction Co.

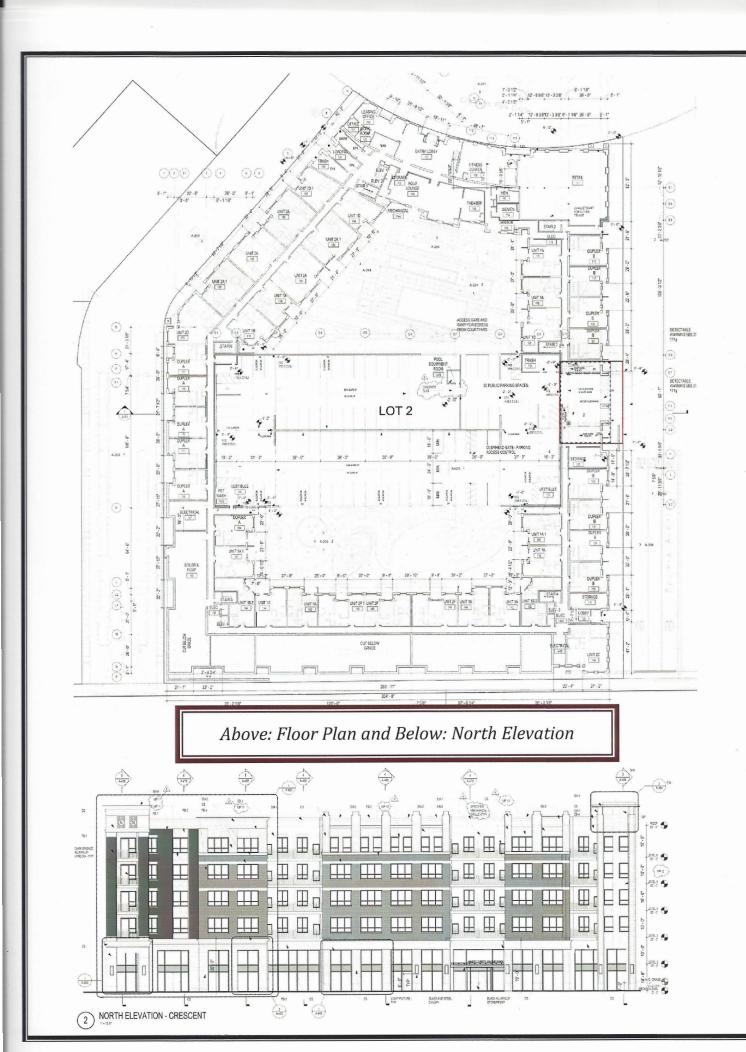
#### MASON CONTRACTOR

J. & E. Duff Inc.

# **Introduction:**

The Village of Orland Park and renowned Midwest multifamily developer Flaherty & Collins Properties are committed to excellence. They united to create a link between the past and the present of the village while developing a new downtown area. McHugh Construction brought their masonry knowledge to the table to save the project 4 million dollars in construction!

masonryadvisorycouncil.org 847-297-6704



The first phase of the Orland Park, IL downtown project is the multifamily Ninety 7 Fifty on the Park with 178 one bedroom apartments and 117 two bedroom apartments for a total of 295 total units. The total floor area for the multifamily building is 486,445 ft<sup>2</sup>. Located near the Metra station and major highways this development is slotted to bring in population to the new downtown.

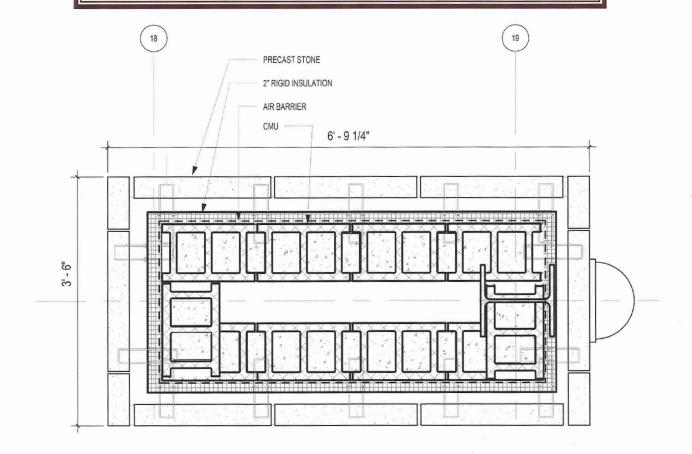
"The System", load bearing concrete masonry with hollow core precast planks provides a fast, efficient, and cost effective building solution for multifamily residences. Ninety7Fifty on the Park in Orland Park, Illinois was a great success story for "The System". These days' general contractors are often brought into the building planning process before architects and act as the owner's advisors for cost savings during construction. General contractors align themselves with different structural systems that have worked in the past for jobs.

At Ninety7Fifty on the Park two general contractors were brought in before McHugh, and each brought with them their preferred structural system. One brought structural steel and another brought precast panels. The final choice for the structural system was load bearing concrete masonry with hollow core precast planks based largely on cost savings versus steel and precast structural systems. The Illinois Structural Masonry Coalition commissioned two independent estimators to analyze the cost comparisons between the precast panels and masonry construction. Their separate estimates both showed masonry savings upwards of \$4,000,000 dollars.

Masonry was shown on this job to shorten the schedule and save time. Masonry does not require prefabrication, and masons can be on a job with a week's notice constructing on a foundation. McHugh chose the structural system to be masonry construction because buildings up to six stories tall can be efficiently constructed for the best cost. Another reason that masonry was chosen over precast was the varying heights and openings in the exterior were difficult to manufacture. Masonry provided the architect with more avenues for creativity in design.

The concrete masonry units were specified in the structural plans to be 4500 psi ASTM C-90 block, Type S Portland cement/lime mortar for load bearing applications and 2500 psi for minimum for the grout. These increases to the individual components in the wall assembly allowed the structural engineer to feel comfortable specifying 2500 psi for the compressive strength of the masonry assemblage, f 'm. A one foot length of wall with 10" grouted cells using 2500 psi compressive strength will support 300,000 lbs at the very least.

Below: CMU pier between large openings in between garage doors



# **CONTRACTOR'S CORNER**

Richard Lauber the owner of J. & E. Duff said "This was a good job because it was structural masonry. It was nice to see that the general contractor was impressed with the ability of masonry to meet and exceed the schedule. The job went up very fast, and we had to wait for the precast planks at the floors levels because we were moving faster than the schedule. It was also nice for us to be responsible for the veneer and the back up because there was no waiting for the steel and the sheathing to go up. There were approximately 245,000 pieces of CMU, 13,500 cubic feet of cast stone and 118,000 utility brick used on this job".

# ORLAND PARK MASONRY ORDINANCE

In the early 20<sup>th</sup> century, Orland Park first began to experience residential development with brick construction in the form of housing that incorporated Tudor, Revival, Four Square and Bungalow architectural styles and accents. The design styles incorporating clay brick were identified as desirable by the community because of the visual appeal, durability and safety brick construction provided. As population increased, the Village officials adopted various fire safety measures to deal with the population growth.

In 1966, Orland Park adopted its first set of masonry standards for new construction by adding language into the building code that required all new commercial and industrial buildings to be constructed of brick, stone or other non-combustible materials. Over the years, Orland Park's masonry standards in the building code have evolved, currently, the Village's architectural design standards require face brick or stone on not less than fifty (50) percent of exterior walls and face brick or stone on ninety (90) percent of each first floor elevation or ground levels of all residential dwellings. All materials for the remaining ground level wall surfaces have to be approved by the

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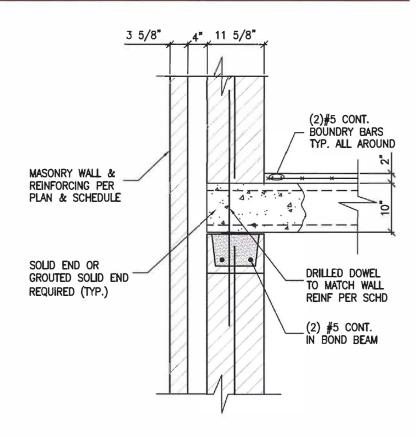
Building Director. Brick or other masonry materials are required on all sides of nonresidential development.

The encouragement for predominance of masonry on new development continues to support Orland Park's long range planning goals of maintaining the value of housing within their neighborhoods. The use of masonry also supports the community's goal of supporting building materials on new development that conserve energy and also encourage the character the Village wishes to preserve within their built environment.

Time lapse of construction

https://www.youtube.com/watch?v=nnSyuzOe57k

Right: Structural
detail of load bearing
12" CMU with
supporting hollow
core precast planks
with a brick veneer





MASONRY WALL SCHEDULE												
TYPE	EXTERIOR BRG.	EXTERIOR BRG.	EXTERIOR BRG.	INTERIOR BRG. WALL	INTERIOR BRG.	INTERIOR BRG.	STAIR/ELEVATOR CORE WALL	SHEAR WALL	EXTERIOR WALL	INTERIOR WALL	INTERIOR WALL	MISC. WALL NOT NOTED ON PLAN
MARK	1\10	22	334	4	(5)	(E)	772	8	9	⊕ {	10	}
HIGH ROOF			8" HOLLOW CMU + AR SPACE, INSUL & 4" NOM, VENER PER ARCH. \$ 60 48" O.C. CTRO IN CMU.		© SASS # HOLLOW CMU #5 © 48° 0.0. CTRO IN CMU.	8" HOLLOW CAU #5 @ 48" O.C. CTRO IN CAU.	8" HOLLOW CAU \$5 © 24" O.C. CTRO IN CAU.				8" HOLLOW CAIU #5 @ 48" O.C. CTRO IN CAIU.	
5TH LEVEL		8" HOLLOW CMU + AIR SPACE, INSUL & 4" NOM. VENEER PER ARCH. \$5 @ 48" O.C. CITO IN CMU.	10" GROUTED SOUID CMU + AIR SPACE, INSUL & 4" NOM. VENEER PER ARCH. \$5 @ 48" O.C. CIRO IN CMU.		e G & HOLLOW CAUL	10" HOLLOW CMU #5 to 48" O.C. CTRO IN CMU.	<b>0</b> (7) 8" НОЦОМ САЛИ #5 <b>0</b> 24° О.С. СТВО IN САЛИ.		8" HOLLOW CMU	B" HOLLOW CANU \$5 @ 48" O.C. CTRO IN CANU.	10" HOLLOW CMU #5 @ 48" O.C. CTRO IN CAU.	
4TH LEVEL	8" HOLLOW CANU + AR SPACE, INSUL & 4" NOM, VENEER PER ARCH. \$ 0 48" O.C. CIRD IN CANU.	8" SOLID CIAU + AR SPACE, INSUL & + NOM, YENEER PER ARCH. \$5 @ 48" O.C. CIRD IN CAUL.		8" HOLLOW CANU #5 @ 48" O.C. CTRO IN CANU.								
3RD LEVEL												
2ND LEVEL	ID CANU ID LAUL & PER ARCH. TRO IN CANU.	ID CMU VIL & VIL AR PER AGCH.	LID CANU NUL & PPER ASCH. STRO IN CANU.	лко дт	ONO ON	ONO GIV	п сэмп	СМ	пр сил	ONO ONO	OUD CANU	лио очл
GROUND LEVE REMARKS		TO GROUTED SOLID CANU  TO GROUTED SOLID CANU	23   24   24   24   24   24   24   24	10° GROUTED SOLID CANU \$5 0 48° 0.6. CTRD IN CANU.	10 GROUTED SOUD CMU #5 @ 48" O.C. CTRD IN CMU.	12" GROUTED SOUID CMU #5 @ 46" O.C. CTRO IN CANU.	8" GROUTED SOULD CANU \$5 0 24" 0.0. CTRD IN GAU.	6" GROUTED SOLID CMU #5 0 24" O.C. CTRO IN OMU.	8" GROUTED SOULD CANU \$5 0 48" 0.0. CTRO IN CAU.	8 GROUTED SOUID CAUU #5 @ 48 0.0. CTRD IN CAUU	10° GROUTED SOULD CMU.	SEE PLANS FOR LOCATIONS & EXTENTS

#### MASONRY NOTES:

PROVIDE FULL MORTAR BED (FMB) FOR ALL MASONRY WALLS

CONTINUE REINFORCING IN SCHEDULE UP THROUGH PARAPETS.

SEE DETAILS ON THIS SHEET FOR ADDITIONAL MASONRY REINFORCING.

PROVIDE DOWELS TO CONCRETE TO MATCH WALL REINFORCING.

PROVIDE ADDITIONAL (1)#5 VERTICAL BAR AT EACH SIDE OF OPENINGS UP TO 3"-0" WIDE, PLACED 18" MAX. FROM EDGE OF OPENING .

PROVIDE ADDITIONAL (2)#5 VERTICAL BARS EACH SIDE OF OPENINGS GREATER THAN 3"-0"WIDE, PLACED 16" MAX. FROM EDGE OF OPENING (TYP. U.N.O.)

PROVIDE ADDITIONAL (1)#5 VERTICAL BAR AT EACH MASSNRY WALL END, PLACED 8" MAX. FROM END OF WALL THIS IS APPLICABLE EACH SIDE OF MOVEMENT JOINTS.

ALL VERTICAL BARS TO BE GROUTED SOLID IN CMU CORES.

PROVIDE 9 CAGE, 2-WIRE, TRUSS TYPE CALVANIZED JOINT REINFORCING VERTICALLY AT 16" O.C., MAX. SEE ARCH DRAWINGS AND SPECS FOR COORDINATION WITH VENEER ANCHORAGE. PROVIDE JOINT REINFORCING WITHIN FIRST TWO BED JOINTS ABOVE AND BELOW ALL WALL OPENINGS AND EXTEND MINIMUM 24" PAST THE EDGE OF SUCH OPENINGS. PROVIDE JOINT REINFORCING WITHIN 8" OF THE TOP OF ALL WALLS.

'SOLID CMU' MAY BE SOLID GROUTED HOLLOW UNITS OR MINIMUM 76% SOLID UNITS.

SEE 1/8" PLANS FOR LOCATIONS OF MASONRY PIERS.

ALL VERTICAL OR HORIZONTAL DEFORMED REINFORCING IS TO BE LAPPED A MINIMUM OF 40 BAR DIAMETERS WHERE LAPS ARE REQUIRED.

ALL NON-BEARING MASONRY WALLS TO BE REINFORCED WITH #4048", MAX. PLUS 1 ADDITIONAL BAR AT ALL CORNERS, ENDS AND OPENINGS. PROVIDE 9 GA. JOINT REINFORCING AT 16", MAX. GROUT ALL BELOW GRADE CMU CORES SOLID.

Masonry wall schedule and general notes

